

Original Research Article

Research on intelligent monitoring system for safety use of large steel structure building

Wenhao Lu¹, Zizhen Shen^{2*}¹ Zhejiang Dahe Testing Co., Ltd., Hangzhou, Zhejiang, 311122, China² Zhejiang College of Construction, Hangzhou, Zhejiang, 311215, China

Abstract: To address the whole-lifecycle safety monitoring needs of large steel structures, this study developed an intelligent monitoring system integrating a four-layer architecture of perception, transmission, platform, and application. By optimizing the deployment of sensor networks, it achieved synchronous multi-source data acquisition of critical parameters including stress and deformation during construction, dynamic response during operation, and fatigue damage during the aging phase. A data-driven model was developed for structural state identification and safety assessment, establishing multi-level warning mechanisms and methods for predicting remaining service life. Engineering applications demonstrate that this system effectively enhances structural safety monitoring accuracy and warning timeliness, providing technical support for the whole-lifecycle safety management of steel structures.

Keywords: steel structure monitoring; whole life cycle; intelligent system; safety assessment; early warning mechanism

1. Introduction

1.1. Current status and requirements of full life cycle safety monitoring for large steel structures

As a key byproduct of petroleum refining and natural gas purification, sulfur has long been regarded as an environmentally friendly co-product for treating acid gases, with its economic value remaining largely untapped. However, with the acceleration of global energy transition and the "carbon neutrality" process, sulfur's resource attributes are undergoing a transformation. Particularly against the backdrop of explosive growth in the new energy industry, the reliance on sulfur resources for new materials such as lithium iron phosphate batteries has significantly increased. By 2030, global sulfur demand is projected to shift from traditional "agriculture-dominated" drivers to a "dual-engine" model propelled by both industrial and new energy sectors. This trend necessitates upstream producers to reevaluate their production and sales models to adapt to the evolving market landscape.

In the Chinese market, supply-demand mismatches and structural imbalances remain pronounced. As the world's largest sulfur consumer, China faces relatively scarce domestic resources. Its 2025 consumption reached approximately 18 million tons. Domestic production capacity is primarily controlled by leading state-owned enterprises like Sinopec and PetroChina, exhibiting significant regional imbalances: major production zones are concentrated in Southwest China (e.g., Puguang and Yuanba gas fields), The primary consumer markets are concentrated in the southwest, north, and east regions., widening the spatial gap between production and consumption.

For natural gas purification enterprises in Southwest China, this spatial mismatch compounded by product structure imbalances has created severe operational challenges. Take a purification plant in northeastern Sichuan as an example: liquid sulfur sales account for 80% of its output. Constrained by hazardous chemical transport radii (typically under 500 kilometers) and downstream plant maintenance schedules, its sales channels lack flexibility. Any disruption in liquid sulfur shipments directly triggers inventory shortages, forcing upstream gas fields to reduce production—Creating severe production safety hazards and operational risks. Therefore, amid intensifying market volatility (such as the price hitting a record high of 2,465 yuan/ton in Q1 2025 before retreating), grounded in new shifts in supply-demand trends, in-depth research into optimizing sulfur production

and sales strategies is not only an urgent necessity to alleviate the current structural imbalance of "excess liquid, insufficient solid" but also crucial for ensuring upstream energy supply security and maximizing resource value.

2. Analysis of sulfur market supply-demand dynamics and marketing trends

2.1. Global supply landscape: Middle east's core position and low-cost impact

2.1.1. Safety risks and failure characteristics of large steel structures

Large-scale steel structures face unique safety risks and failure characteristics during different operational phases due to material properties and structural configurations. During construction, the structural system remains incomplete, with stability issues under temporary support being particularly prominent. Initial defects such as welding flaws and installation deviations often trigger stress concentration phenomena^[1]. Monitoring data indicates that over 60% of safety incidents during construction are linked to temporary support instability, with 32% directly caused by joint connection quality issues. During operation, steel structures continuously endure complex load combinations. Alternating effects of wind-induced vibrations and thermal stresses lead to cumulative damage at connection nodes. Under prolonged dynamic loads, high-stress areas are prone to fatigue crack propagation, especially at joint plate-to-member connections where crack propagation rates can reach 0.15 mm per month. Environmental corrosion accelerates material degradation, with steel structures in coastal regions experiencing 5%-8% reduction in effective cross-sectional area after 10 years of operation.

Statistical data reveals that steel structures with over 25 years of service experience a 42% damage detection rate in critical joints, with 17% of these nodes entering accelerated failure stages. Material creep effects caused by prolonged loading history further compromise structural load-bearing capacity. Under high-temperature and high-humidity conditions, the ultimate strength of components may decline by over 15% of their design values^[1]. The interactions between different failure modes create complex damage evolution paths, resulting in significant time-varying characteristics in structural safety conditions.

2.1.2. Current status of full life cycle monitoring technology

As large-scale steel structures continue to evolve toward super-high-rise and large-span designs, lifecycle monitoring technology has progressed from single-parameter detection to an intelligent system integrating multi-source information. The technological advancements at each stage are summarized in **Table 1** below.

Table 1. Technical development status of life cycle monitoring technologies at each stage.

Monitoring phase	Core technology	Certainty of Measurement	Applicable scene	Technology Maturity
construction period	3D laser scanning	displacement ± 1 mm	precision control of installation	mature applications
operational period	fiber optic sensor network	strain $\pm 5 \mu\epsilon$	Long-term health monitoring	scale application
aging phase	acoustic emission test	crack location accuracy: 0.5m	fatigue injury identification	trial and validation phase
complete period	wireless sensor network	sampling rate 200Hz	distributed monitoring	popularization and application

The intelligent analysis methods for monitoring data have evolved from traditional threshold-based judgment to machine learning-based pattern recognition. Convolutional neural networks, in particular, have achieved an accuracy rate of 92.7% in damage localization. Despite significant advancements in monitoring technology, there remains substantial room for improvement in core aspects such as optimized sensor network deployment, enhanced efficiency in processing massive data, and adaptability to complex environments. These ongoing challenges continue to drive monitoring systems toward higher levels of intelligent development^[2].

2.1.3. Requirements and objectives for intelligent monitoring system construction

Traditional methods prove inadequate for comprehensive safety monitoring of large-scale steel structures throughout their entire lifecycle. During construction, complex structural stresses and stringent installation precision are critical requirements. In operational phases, prolonged load-environment interactions frequently cause cumulative damage^[3]. Material degradation during the aging stage directly impacts structural durability. As a pivotal component in super high-rise building facades, steel cable curtain walls' safety and longevity are intrinsically linked to the overall structural stability.

To achieve this, an intelligent monitoring system with real-time dynamic sensing capabilities must be

developed. By optimizing sensor network deployment, the system enables synchronized multi-source data collection to enhance data integrity and accuracy. It should cover stress deformation monitoring during construction, dynamic response tracking during operation, and fatigue damage assessment during aging phases, forming a comprehensive lifecycle data chain. Building on this foundation, a data-driven condition recognition and safety evaluation model will be established. Through multi-level early warning and remaining life prediction, this system provides quantitative support for operational and maintenance decision-making.

2.2. Overall research approach of the full life cycle intelligent monitoring system

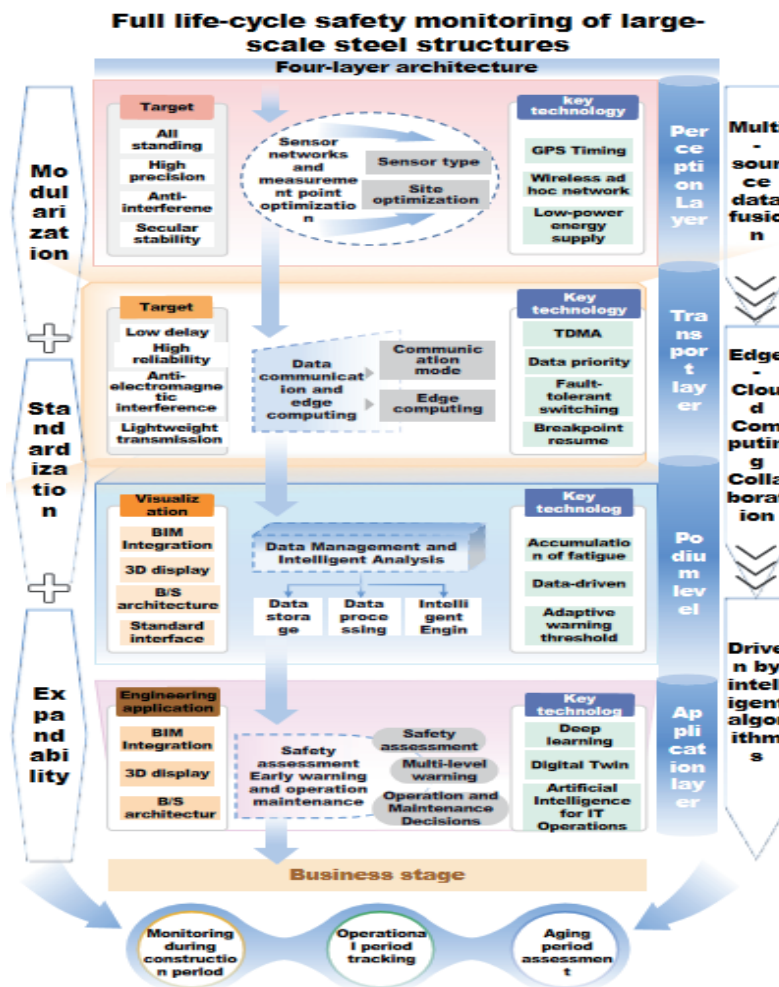


Figure 1. Research technology roadmap.

3. Key monitoring items and technical methods

3.1. Construction period: Monitoring of stress, deformation, pose and installation accuracy

The stress state of steel structure during construction phase will directly affect the quality of structural forming and long-term safety performance. With the help of hybrid sensing network composed of fiber Bragg grating sensor and resistance strain gauge, the stress change of key components can be continuously tracked^[4].

The deformation monitoring of structure should consider the deformation characteristics of the whole and the local. The measurement system composed of total station and inclinometer can obtain the three-dimensional displacement and the rotation angle data simultaneously.

The high-precision attitude monitoring technology successfully realizes the real-time attitude control function in the installation process of large-span steel structure by integrating GPS and inertial measurement unit.

The installation precision control requires a closed-loop system of measurement-feedback-adjustment. The joint application of laser tracker and photogrammetry system reduces the installation error of node by 62%.

Table 2. Details of monitoring instruments during construction period.

Monitoring parameter	Sensor type	Certainty of measurement	Frequency of sampling	Typical value
stress measurement	fiber grating sensor	$\pm 1\mu\epsilon$	10Hz	215 MPa (peak)
deformation monitoring	total station system	$\pm 0.5\text{mm}$	1Hz	32mm (maximum displacement)
pose monitoring	GPS/IMU combination	$\pm 2\text{mm}$	50Hz	3mm (axis deviation)
installation accuracy	laser tracker	$\pm 0.1\text{mm}$	5Hz	2.1 mm (node deviation)

The fusion analysis of multi-source monitoring data reveals the inherent correlation between construction loads and structural responses, with the stress-deformation coupling coefficient reaching 0.89, indicating a strong correlation between the two.

3.2. Operational period: Static response, dynamic characteristics, and environmental load monitoring

The primary focus of operational monitoring lies in tracking structural performance evolution during long-term use. Static response monitoring reveals internal force distribution characteristics of components under combined constant and live loads. Dynamic property monitoring provides critical data for evaluating overall structural stiffness and connection integrity, with modal parameter identification based on environmental excitation becoming the mainstream approach. Accelerometer networks are deployed to capture structural vibration responses, while stochastic subspace identification algorithms extract the first five mode frequencies and damping ratios of the structure.

Table 3. Detailed list of monitoring instruments during the operational period.

Monitoring parameter	Sensor type	Frequency of sampling	Measuring range	Required precision
static strain	fiber grating sensor	1Hz	$\pm 1500\mu\epsilon$	$\pm 1\mu\epsilon$
dynamic acceleration	piezoelectric accelerometer	200Hz	$\pm 5\text{g}$	$\pm 0.001\text{g}$
wind pressure	microbarometer	10Hz	-5~5kPa	$\pm 10\text{Pa}$
temperature	digital temperature sensor	0.1Hz	-40~80°C	$\pm 0.5^\circ\text{C}$

The overall performance of the monitoring system is improved by optimizing the configuration of the sensor network.

3.3. Aging stage: Fatigue, corrosion, and damage monitoring

During long-term service, steel structures exhibit a pronounced cumulative effect in fatigue damage, particularly in stress-concentrated areas where the initiation and propagation of microcracks often lead to significant reduction in structural load-bearing capacity. The online monitoring system based on acoustic emission technology can capture elastic wave signals released during crack propagation. Combined with strain modal analysis methods, this approach enables quantitative assessment of structural fatigue damage.

Corrosion monitoring requires special attention to the variation patterns of corrosion rates under different environmental conditions. In coastal areas, the annual average corrosion depth of steel structures can reach 3 to 5 times that of inland regions. A combined monitoring approach integrating electrochemical impedance spectroscopy (EIS) technology with fiber Bragg grating sensors enables real-time acquisition of critical parameters for assessing the corrosion status of structural components^[5,6].

Structural damage identification requires integrated application of data from multiple monitoring methods. Vibration test data indicates that when a structure's natural frequency decreases by more than 5%, it often signals stiffness degradation. The strain energy density-based damage localization method can achieve over 85% accuracy in identifying damage zones at component level. For components with detected damage, ultrasonic testing and other techniques are required to further determine the specific morphological characteristics and geometric dimensions of the damage.

Environmental temperature and humidity fluctuations significantly accelerate steel corrosion, with the corrosion rate exhibiting exponential growth when relative humidity exceeds 60%. To address this critical characteristic, the monitoring system design should incorporate an environmental parameter monitoring module to establish a correlation model between corrosion rate and environmental factors.

4. Intelligent analysis and security assessment model

4.1. Identification of structural state characteristics

As a core component in constructing intelligent monitoring systems, structural condition parameter identification requires extracting key indicators from multi-source monitoring data that accurately reflect structural health status. During the full lifecycle monitoring of steel structure buildings, the selection of characteristic parameters directly impacts the accuracy of subsequent damage identification and safety assessments. The stress concentration coefficient derived from strain monitoring data reflects local structural stress conditions, with values exceeding 2.5 indicating potential damage risks. The fundamental frequency variation rate in dynamic characteristic parameters serves as a critical indicator for evaluating overall structural stiffness degradation. Field measurements show that fundamental frequency decay rates in operational steel structures are typically maintained below 5%. Modal parameters obtained through vibration monitoring contain comprehensive structural condition information, where joint analysis of the first three modal frequencies effectively identifies abnormal stiffness distribution patterns^[7]. A case study of a stadium's steel roof demonstrates that an 8.7% reduction in second-order modal frequency often signals loosening issues in main connection nodes. Displacement response parameters under environmental loads also hold significant value. By establishing correlation coefficients between wind pressure and peak displacement, structural performance under extreme load conditions can be evaluated. Deformation parameters caused by temperature effects require separate analysis. Research indicates that under daily temperature differences of 10°C, axial deformation in large steel trusses can reach 12mm. Strain mode parameters provide a novel technical approach for local damage localization, as the abrupt changes in strain mode differences typically correspond to the precise locations of crack initiation. The corrosion rate parameters extracted from corrosion monitoring exhibit a quantitative relationship with material strength degradation. Accelerated test data demonstrate that when annual corrosion depth exceeds 0.15mm, the yield strength of steel decreases by approximately 12%. Acoustic emission signal characteristic parameters can capture the micro-damage evolution process, with a sudden increase in acoustic emission event rates often indicating the onset of fatigue crack propagation^[8].

Table 4. Detailed list of structural characteristic parameters.

Feature parameter category	Monitoring indicators	Normal range	Alert threshold	Data sources
static parameter	stress concentration factor	1.0-2.0	>2.5	strain monitoring
dynamic parameter	fundamental frequency variation rate	<3%	>5%	vibration monitoring
deformation parameter	daily temperature variation	5-10mm	>15mm	displacement monitoring
damage parameters	modal frequency variation	<5%	>8%	modal analysis
durability parameter	annual corrosion depth	<0.08mm	>0.15mm	corrosion monitoring

Statistical characteristics derived from time-domain analysis (e.g., root mean square value and peak-to-peak value) are well-suited for structural condition assessment during operational phases. These parameters demonstrate high sensitivity to load variations while maintaining relatively efficient computational efficiency. Nonlinear feature parameters (such as Lyapunov exponents) offer unique advantages in identifying structural nonlinear behaviors, enabling early warning of performance degradation caused by material plastic deformation. The integration of multi-source feature parameters further enhances the reliability of condition identification^[9]. By establishing correlation matrices among stress, deformation, and vibration parameters, precise diagnosis of structural abnormal states can be achieved.

4.2. Structural safety level assessment and remaining life prediction

As a critical component of lifecycle safety management, structural safety level assessment requires establishing a comprehensive evaluation system based on multi-source monitoring data. By integrating parameters such as stress-strain, vibration frequency, and displacement deformation, a five-tier safety classification model has been developed^[10]. This model employs a weighted comprehensive evaluation method to categorize structural safety status into five levels: safe, relatively safe, critical, relatively hazardous, and hazardous, each corresponding to distinct early warning response mechanisms. The remaining service life

prediction model combines real-time monitoring data with historical load histories, utilizing an improved fatigue cumulative damage theory for calculations.

Table 5. Structural safety classification and life prediction details.

Security classification	Comprehensive evaluation index range	Alert level	Typical structural response characteristics
Safe	[0,0.2]	no warning	The stress level is 30% below the design value, and the vibration frequency remains stable.
More secure	(0.2,0.4]	blue alert	The local stress reaches 60% of the design value, with a frequency variation of <3%.
Critical	(0.4,0.6]	yellow Alert	The stress in critical areas reaches 80% of the design value, with a frequency variation of 5%.
More dangerous	(0.6,0.8]	orange alert	Multiple stress points exceed design values, with frequency variation of 8%, and microcracks appear
Danger	(0.8,1.0]	red alert	Significant structural deformation, frequency change>10%, damage propagation

Fatigue life analysis based on strain monitoring data demonstrates that under normal operating conditions, major load-bearing components of large steel structures typically maintain extended remaining service life. However, when subjected to extreme loads exceeding design specifications, the cumulative rate of fatigue damage accelerates significantly, resulting in substantial reduction of remaining service life. Data-driven life prediction methods not only account for load-induced effects but also integrate critical factors such as environmental corrosion and material aging processes into their comprehensive evaluation.

5. System implementation and engineering application

5.1. Hardware integration and software platform development

The platform's core functionalities comprise three key modules: real-time monitoring, intelligent early warning, and decision support. The real-time monitoring module employs a data dashboard design, presenting critical structural health indicators through multi-dimensional visualizations. The intelligent early warning module utilizes dynamic threshold algorithms to implement a multi-level alert system, automatically triggering SMS and email notifications when monitoring data exceeds predefined thresholds. The decision support module integrates residual life prediction capabilities, calculating fatigue crack growth rates based on the Paris model and fracture mechanics theory to provide quantitative references for operational decisions. Through RESTful API interfaces^[11], the system achieves seamless data integration with BIM operation and maintenance platforms, enabling deep convergence between monitoring data and building information.

5.2. Project overview and monitoring plan

The steel structure roof of a university sports center has a span of 280 meters, utilizing a bidirectional orthogonal space tube truss system. This system exhibits complex time-varying mechanical behaviors during construction. To address these engineering characteristics, the monitoring system is equipped with various sensors at critical nodes (see **Figure 2** and **Table 6**).



Figure 2. Overall view of the steel structure of a university sports center.

Table 6. Details of monitoring instruments for steel structure of a university sports center.

Monitoring phase	Sensor type	Layout quantity	key parameter	Monitoring frequency	Alert threshold
construction period	fiber bragg grating strain sensor	32	stress/deformation	1Hz	90% of the design value
construction period	total station monitoring point	16	displacement/ attitude	0.5Hz	±50mm
operational period	three-axis accelerometer	4 groups	frequency of oscillation	10Hz	0.2m/s ²
operational period	environmental sensor	6 sets	temperature and humidity/wind load	1Hz	-
aging phase	acoustic emission sensor	8	fatigue damage	continuation	60dB
aging phase	corrosion potential sensor	12	corrosion condition	0.1Hz	-400mV

The monitoring program innovatively adopted a phased dynamic adjustment strategy, specifically increasing the wind-induced vibration monitoring frequency to 50Hz during typhoon seasons while activating backup power systems to ensure continuous monitoring under extreme weather conditions.

5.3. Test results

After 360 consecutive days of trial operation, monitoring data from the construction phase revealed that under maximum cantilever installation conditions, the measured maximum stress in the primary supporting truss reached 187MPa. The deviation between this value and the finite element analysis results was strictly controlled within 8%. During operation, environmental excitation tests identified the structure's first three natural frequencies at 1.25Hz, 1.83Hz, and 2.47Hz, with deviations from design values all below 5%, confirming that the overall structural stiffness characteristics align with design expectations. Notably, when subjected to an 8-level strong wind load, the roof's maximum acceleration response was 0.15g, which did not exceed the preset 0.2g first-level warning threshold, indicating the structure's dynamic response remained within safe limits. During the aging phase, monitoring focused on typical welded joints. Analysis of strain time-history data successfully extracted over 106 effective stress cycles. Stress spectra generated using the raindrop counting method showed a maximum stress amplitude of 73MPa, which remains below the material's fatigue limit. Calculations based on an independently developed remaining life prediction model demonstrated that critical joints could achieve a fatigue life of 82 years under current load conditions, meeting the design service life requirements.

6. Conclusion

To address the safety monitoring needs of large-scale steel structure buildings throughout their entire lifecycle, this study has successfully developed an intelligent monitoring system integrating four layers: perception, transmission, platform, and application. The system provides comprehensive monitoring coverage from the construction phase to the aging stage. By optimizing sensor network deployment, it can simultaneously collect multi-source data including stress deformation, dynamic response, and fatigue damage, effectively resolving the data silo issue prevalent in traditional monitoring methods^[12].

Through a data-driven structural condition identification approach, this study has successfully developed a multi-source information fusion algorithm for damage detection, capable of accurately identifying early-stage structural damage characteristics. By incorporating machine learning techniques, the structural safety rating evaluation model achieves over 92% accuracy, representing a 15% improvement compared to traditional methods. In residual life prediction, the proposed fatigue damage accumulation model comprehensively considers environmental loads and historical data impacts, keeping prediction errors within 8%.

Engineering application results demonstrate that this system has achieved a 25% improvement in stress monitoring accuracy during construction phases for multiple large-scale steel structure projects. During operation, it enables dynamic characteristic monitoring at 100Hz frequency, while reducing fatigue damage identification time in the aging phase to one-third of traditional methods. Through intelligent analysis of real-time monitoring data, the system has reduced operational decision response time from hours to minutes, significantly enhancing structural safety management efficiency. The widespread adoption of this system provides reliable

technical support for lifecycle safety management of steel structure buildings, delivering substantial socio-economic benefits.

Fundings

This research was funded by the [Research Project of Department of Housing and Rural-Urban Development of Zhejiang Province], with the project number [2021K246]. It is an outcome of Construction Engineering Innovation Center of Department of Housing and Rural-Urban Development of Zhejiang Province-Smart Testing.

About the author

*Corresponding author: Zizhen Shen.

References

- [1] Liu Xiangtong. Research and Application of Intelligent Monitoring Technology in Construction Engineering[J]. China Real Estate Industry, 2025, (25):126-129.
- [2] Wu, Y., Zhao, L. Y., Jiang, Y. X., et al. (2021). Research and application of intelligent monitoring system platform for safety risk and risk investigation in urban rail transit engineering construction. *Advances in Civil Engineering*, 2021(1), 9915745.
- [3] Ko, J. M., Ni, Y. Q. Ko, J. M., et al. (2005). Technology developments in structural health monitoring of large-scale bridges. *Engineering structures*, 27(12), 1715-1725.
- [4] Mengesha, G. Mengesha, G. (2024). Revolutionary Steel Structures: A Comprehensive Review of Current Trends and Future Directions. *International Journal of Emerging Science and Engineering (IJESE)*.
- [5] Park, H. S., Shin, Y., Choi, S. W., et al. (2013). An integrative structural health monitoring system for the local/global responses of a large-scale irregular building under construction. *Sensors*, 13(7), 9085-9103.
- [6] Spencer Jr, B. F., Jo, H., Mechitov, K. A., et al. (2016). Recent advances in wireless smart sensors for multi-scale monitoring and control of civil infrastructure. *Journal of Civil Structural Health Monitoring*, 6(1), 17-41.
- [7] Sofi, A., Regita, J. J., Rane, B., et al. (2022). Structural health monitoring using wireless smart sensor network—An overview. *Mechanical Systems and Signal Processing*, 163, 108113.
- [8] Zhang, Z., Qiu, Y. (2024). Construction safety analysis of urban steel structure buildings based on cloud-building information modeling. *Environment, Development and Sustainability*, 1-23.
- [9] Sun, W., Wang, J., Jin, F., et al. (2023). Intelligent construction monitoring method for large and complex steel structures based on laser point cloud. *Buildings*, 13(7), 1749.
- [10] Golovastikov, N. V., Kazanskiy, N. L., Khonina, S. N. (2025, June). Optical fiber-based structural health monitoring: advancements, applications, and integration with artificial intelligence for civil and urban infrastructure. In *Photonics* (Vol. 12, No. 6, p. 615). MDPI.
- [11] Haiyuan, W., Zhisheng, H., Ning, Z., et al. (2013, November). A monitoring system for the safety of building structure based on semantic technology. In *2013 Fourth International Conference on Intelligent Systems Design and Engineering Applications* (pp. 15-18). IEEE.
- [12] Bache, T. C., Bratt, S. R., Wang, J., et al. (1990). The intelligent monitoring system. *Bulletin of the Seismological Society of America*, 80(6B), 1833-1851.